

Landfill Liner Failure: An Open Question for Landfill Risk Analysis

Alberto Pivato

IMAGE, Department of Hydraulic, Maritime, Geotechnical and Environmental Engineering, University of Padua, Padova, Italy.
Email: pivato@idra.unipd.it, alberto.pivato@libero.it

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ABSTRACT

The European Union Landfill regulations (1999/31/EC) are based on the premise that technological barrier systems can fully contain all landfill leachate produced during waste degradation, and thus provide complete protection to groundwater. The long-term durability of containment systems are to date unproven as landfill liner systems have only been used for about 30 years. Many recent studies have drawn attention to some of the deficiencies associated with artificial lining systems, particularly synthetic membrane systems. Consequently, failure modes of landfill liners need to be quantified and analysed. A probabilistic approach, which is usually performed for complex technological systems such as nuclear reactors, chemical plants and spacecrafts, can be applied usefully to the evaluation of landfill liner integrity and to clarify the failure issue (reliability) of liners currently applied. This approach can be suitably included into risk analysis to manage the landfill aftercare period.

Keywords: Landfill Liners, System Reliability, Risk Analysis, Landfill Aftercare Period

1. Introduction

In the last decades the contained landfill has been developed, installing liners (mineral and synthetic) and collecting gas and leachate emissions.

However, many researches have found that the lining system has limited (10 - 30 years) duration. When liners fail, a variety of compounds whose concentration may be above the acceptable level (table values) spread into the environment.

The uncontrolled emissions depend on the long term behaviour of chemicals in the landfill and on the typology of liner failure. **Figure 1** shows a potential scenario of contamination constituted by a biodegradable organic chemical leakage. The uncontrolled emissions to the environment over the time is the sum of two opposite processes: a long-time degradation of chemicals in the landfill and a short-time increase of leachate leakage due to liner failure.

The first process is generally modelled by a first order kinetic such as:

$$C_1(t) = C_0 \cdot e^{-k \cdot t}$$

where: $C_1(t)$ is the concentration of the contaminant in the leachate (mg/m^3); C_0 is the initial peak concentration of the contaminant in the leachate (mg/m^3); t is the simulation time; k is a kinetic constant describing the rate of

decrease of the chemical. This value can be expressed also by the half time ($T_{1/2}$): $k = \ln 2 / T_{1/2}$.

The second process depends on many variables such as the leachate head, the liner layer and the liner performance. Many analytical models have been proposed and all show an initial period in which the leakage is very low because the the containment system is expected to function adequately. The results are in term of leachate quantity by time (m^3/day) that emigrates from the landfill to the environment.

The problem consists in the fact that the potential emissions from landfills (biogas and leachate) can last for a very long time (centuries), more than the barriers (liners).

In order to control long term environmental impact and guarantee landfill sustainability an approach based on the risk evaluation of long term emissions should be assessed; this is mainly correlated to the chemical degradation into the landfill and to the barrier (e.g. liner) performance. However, the Landfill regulations in Europe state that aftercare must continue for almost 30 years after the site has been closed independently to the landfill risk at that time. This is a bureaucratic term and after 30 years the landfill will be a contaminated soil, no longer financially supported by a waste fee. The operations planned for this phase consist only in monitoring and

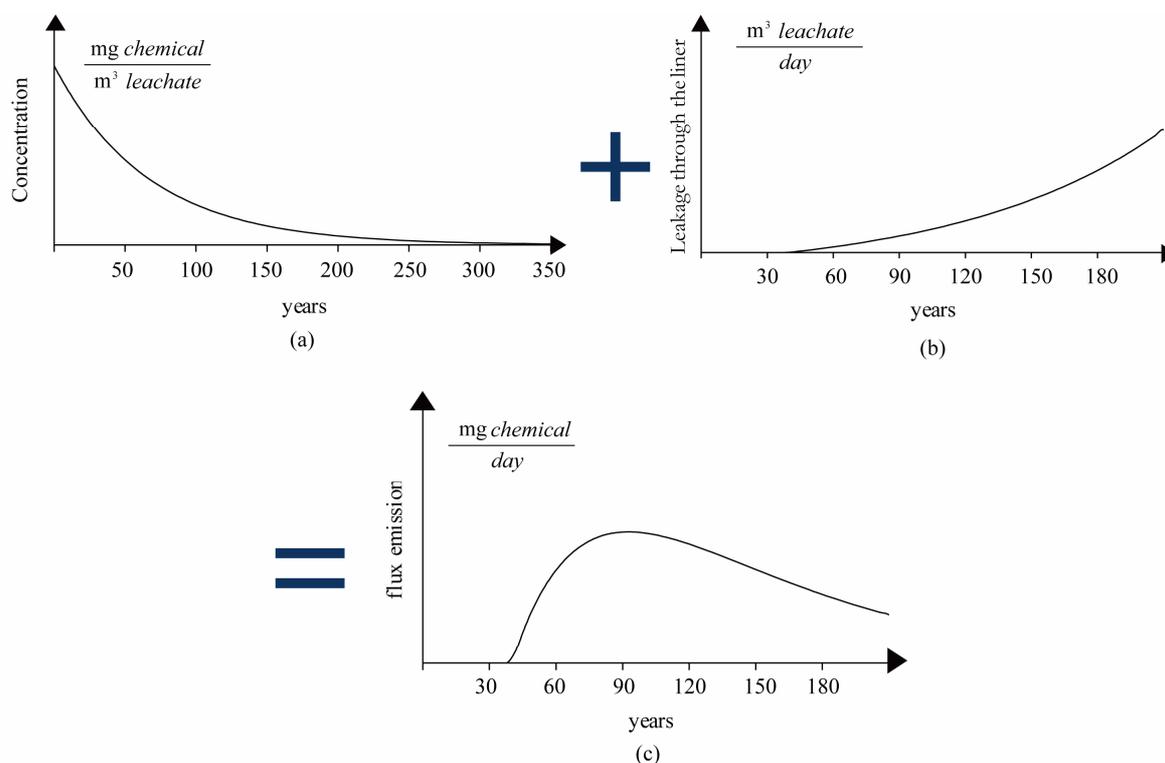


Figure 1. Qualitative long-term behaviour of uncontrolled emissions over the time (c) due to two opposite processes: (a) a long-time degradation of chemicals in the landfills; (b) a short-time increase of leachate leakage due to liner failure.

maintenance activities. The implication is that monitoring will be discontinued after 30 years assuming the landfill is stable and no longer represents a threat to the environment.

There is increasing recognition that time alone is an inadequate indicator of whether or not a landfill may be regarded as adequately stabilized.

In this context landfill risk analysis applied to after-care period is obtaining interest by scientific community.

The risk involved with the release of contaminants present in waste has usually been addressed by assessing the human/environmental effects that may result from human/environmental exposure to a conservative scenario. Risks are analysed due to the fact that contaminants have been released from the waste bulk into the adjacent environmental compartments. Historically, waste was simply dumped into a pit in the ground; no engineered measures were applied (which could be failure analysed). For modern landfills, such as those provided with currently available containment technology, the risk assessment procedure needs to include assessment of source-released risk that would occur if the liner failed.

2. The Use of Reliability Studies

Containment system failure can be defined as any egress of substances (any release) from the liner when the leachate head is at least 30 cm. This definition is in accordance with the reliability studies of Rodic-Wiersma and Goossens [1]. However, in practice there is no long-term experience regarding modern landfill technology from which to draw conclusions about long-term performance. Certainly, the containment system applied cannot be expected to function for an indefinite period of time. Reliability study principles should be applied not only to the overall design but also to the details of individual materials and their methods of installation. Some authors have proposed a ranking list of the most probable causes of failure by using 'pairwise comparison' technique [2].

The reliability of liner systems is the aptitude to carry out specific functions, when used in the expected conditions. The reliability of liners, and consequently of their failure, depends on several events, each characterized by an actual probability.

Typical causes of failure of landfill bottom liners are:

- Bad geomembrane seams and/or clay compaction;
- Installation damage;

- Not safeguarding liner in operation;
- Pipes penetrating liner;
- Clogging of the leachate collection and removal system;
- Geotechnical failure;
- Unanticipated chemical attack;
- Breach by vertical pipes.

The reliability evaluation can be carried out with two different approaches.

The first is deductive analysis, which analyses a series of similar historical failure events. A considerable amount of information on different installations should be collected and divided into the better comparable categories according to the characteristic elements. For example, a landfill with only a clay liner on the bottom should be included in the group that contains the same containment system. Once the reliability for a set of landfills with similar features has been estimated, a statistical estimator can be defined and extended to the whole group.

The comparison is always subject to approximation, due to the diversification of the boundary conditions: the geology of the sites, the environmental conditions, the design and the materials, etc. In a comprehensive evaluation, it is also important to consider the analogies in the different working conditions. These precautions are needed in order to develop a statistical study that produces results consistent with the aforementioned reliability definition as well as reduce the inevitable approximations and uncertainties in this type of comparison.

A more adaptable and reliable method is predictive analysis. This analysis entails knowledge of failure probability of the individual elements (subsystems) and combines them with an appropriate probabilistic analysis to define the reliability of a more complex system. A standardized procedure is "Fault Free" analysis, which is used in the Netherlands and in other countries to predict the aftercare period cost [3].

Aftercare period costs are the ones connected to the operations planned for this phases and consist only in monitoring and maintenance activities:

- Cap maintenance and monitoring;
- Leachate recirculation operation and maintenance (where permitted!);
- Leachate collection system operation and maintenance;
- Landfill gas collection system maintenance and monitoring;
- Landfill gas migration control and monitoring;
- Groundwater and surface water monitoring;
- Security and grounds maintenance.
- The leakage of a bottom liner, *i.e.* the failure of the

barrier, is caused by one or a set of system components generating failure events. The environment, plant personnel, aging of materials etc. can influence the system only through its components. As proposed by Henley and Kumamaoto [4] we distinguish different component failures:

- A primary failure is defined as the component being in the non-working state for which the component is held accountable. A primary failure occurs under inputs within the design envelope, and component natural aging is responsible for such failure. Among other aspects, the aging of the components in the liner depends on the chemical composition of the leachate and on the high temperature due to the exothermic reactions inside the landfill.
- A secondary failure is the same as a primary failure except that the component is not held accountable for the failure. Past or present excessive stresses placed on the component are responsible for secondary failure. Examples are environmental stresses (geological assessment, uncontrolled groundwater infiltration, high leachate head, etc.), human error such as if personals break the components (installation damage, bad compaction of clay liner, etc.).
- A command fault is defined as the component being in the non-working state due to improper control signal or noise (failure of pump signal to extract leachate, etc.).

This subdivision is essential in order to properly collect failure data for reliability studies.

In the present work, basic events related to system components with binary states, *i.e.*, normal state and failed state will be quantified first. The quantification is then extended to components having plural failure modes.

3. Single Failure Mode Analysis

We assume that at any given time a liner system is either functioning normally or failed, and that the component state changes as time evolves (**Figure 2**). It is assumed that the component changes its state instantaneously when the normal to failed transition takes place. The transition to the failed state is failure and the failed state continues forever if the component is non-repairable (as generally is the case of a landfill liner).

The time failure is defined as the interval of time between the moment the barrier system is put into operation (including all the elements composing it) and its failure. This interval is generally a stochastic variable ($x \geq 0$). The distribution $F(t) = P\{\mathbf{x} \leq t\}$ is the probability that the system fails prior to time t , assuming that the system has been in function since $t = 0$. The system reliability is expressed by:

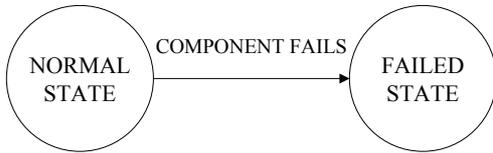


Figure 2. Transition diagram of component state.

$$R(t) = 1 - F(t) = P\{x > t\}$$

The mean time of failure is the mean of the variable x [5]. Since $F(x) = 0$ for $x < 0$, we conclude that:

$$MTTF = \int_0^{\infty} R(t) dt$$

Probability that the system functioning at time t fails prior to time $x > t$ equals:

$$F(x/x > t) = \frac{P\{x < x, x > t\}}{P\{x > t\}} = \frac{F(x) - F(t)}{1 - F(t)}$$

differentiating with respect to x :

$$f(x/x > t) = \frac{f(x)}{1 - F(t)}$$

The product $f(x/x > t) dx$ equals probability that the system fails in a time interval $(x, x + dx)$, assuming that it functions at time t . The conditional density $f(x/x > t)$ is a function of x and t . Its value at $x = t$ is a function of t only. This function is denoted as $\lambda(t)$ and is called the failure rate:

$$\lambda(t) = f(t/x > t) = \frac{f(t)}{1 - F(t)}$$

The failure rate is the probability that the component experiences a failure per unit time at time t , given that the component is in normal state at time zero and is normal at time t . A suitable model is the one proposed by Herz [6] developed for water mains. He proposed a failure probability distribution density function based on the principles that had originally been applied to population age classes or cohorts. The probability density ($f(t)$), failure rate ($\lambda(t)$) and failure probability ($F(t)$) functions are:

$$f(t) = \frac{(a+1) \cdot b \cdot e^{b(t-c)}}{[a + e^{b(t-c)}]^2}$$

$$\lambda(t) = \frac{b \cdot e^{b(t-c)}}{a + e^{b(t-c)}}$$

$$F(t) = 1 - \frac{(a+1) \cdot b \cdot e^{b(t-c)}}{a + e^{b(t-c)}}$$

where a is the aging factor (year-1); b is the failure factor (year-1); and c is the resistance time (years).

4. System Reliability Analysis

The problem considered above strictly involves a single failure mode, defined by a single failure state. Many physical systems that are composed of multiple components can be classified as series connected systems or parallel-connected systems, or a combination of both. More specifically, the failure events (eg. in the case of multiple failure modes) may also be represented as events in series (union) or in parallel (intersection) (Figure 3).

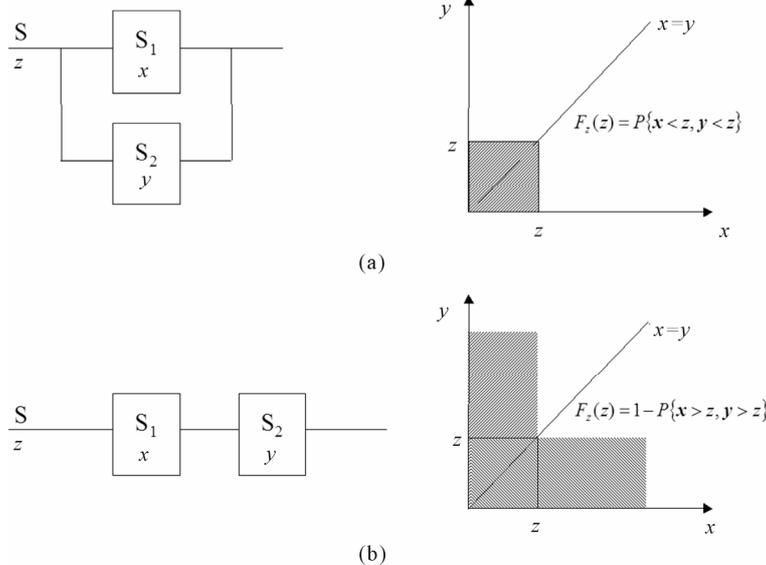


Figure 3. Interconnection of systems: (a) parallel; (b) series. The figures on the right show the regions in the x, y space that satisfy the probability conditions.

We can assume that a landfill is constituted by several cells (system in series) and each cell is provided with a liner with more elements (system in parallel). Each cell will function as long as at least one liner functions and the complete landfill system will function as long as all the cells function.

Two systems S_1 and S_2 , with failure times respectively x and y , can be connected in parallel or series, making a new system with failure time z (Figure 3). In the case of system in parallel, the system S fails when all the subsystem fails and the following expression is used:

$$z = \max(x, y)$$

If the two systems are independent, then:

$$F_z(z) = P\{x < z, y < z\} = F_x(z) \cdot F_y(z)$$

In the case of system in series, the system S fails when at least one subsystem fails and the following expression is used:

$$z = \max(x, y)$$

If the two systems are independent, then:

$$F_z(z) = 1 - P\{x > z, y > z\} \\ = F_x(z) + F_y(z) - F_x(z) \cdot F_y(z)$$

We can assume that a landfill is constituted by several cells (system in series, Figure 4) and each cell is provided with a liner with more elements (system in parallel, Figure 5). Each cell will function as long as at

least one liner is functioning and the complete landfill system will function so long as all the cells are functioning.

Complex liner systems involve multiple failure modes, in which the occurrence of any one of the potential failure modes will constitute failure or non-performance of the system or component. A systematic scheme, such as a Fault Tree for identifying all potential failure modes, may be required.

4.1. Fault Tree Analysis

A Fault Tree is widely used to assess the failure of a “Technological System”. Firstly, the Technological System for which the analysis to be performed is defined. Then, a system failure event is specified (this is called Top Event) and a “backwards” analysis is conducted to identify all possible chains of events that could lead to the given end point. In doing so, individual basic events are identified which may lead to the top event alone or in combination with others. It makes use of a codified symbology for the events and for those decision-making structures (Logical Gate). A summary of such symbology is collected in Table 1.

The fundamental logic gates are AND and OR. The logic functions and indicates that an event occurs only if all of the sub-events take place simultaneously. The logic functions or indicates that an event occurs only if at least one of the sub-events is verified, independently from others.

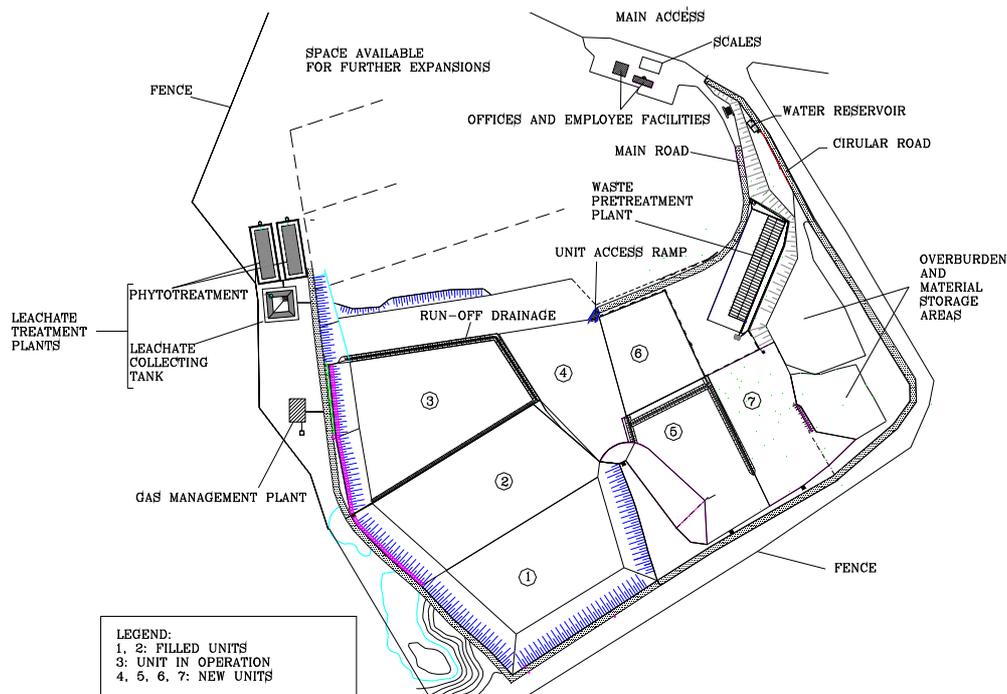


Figure 4. Example of a landfill with several cells (system in series).

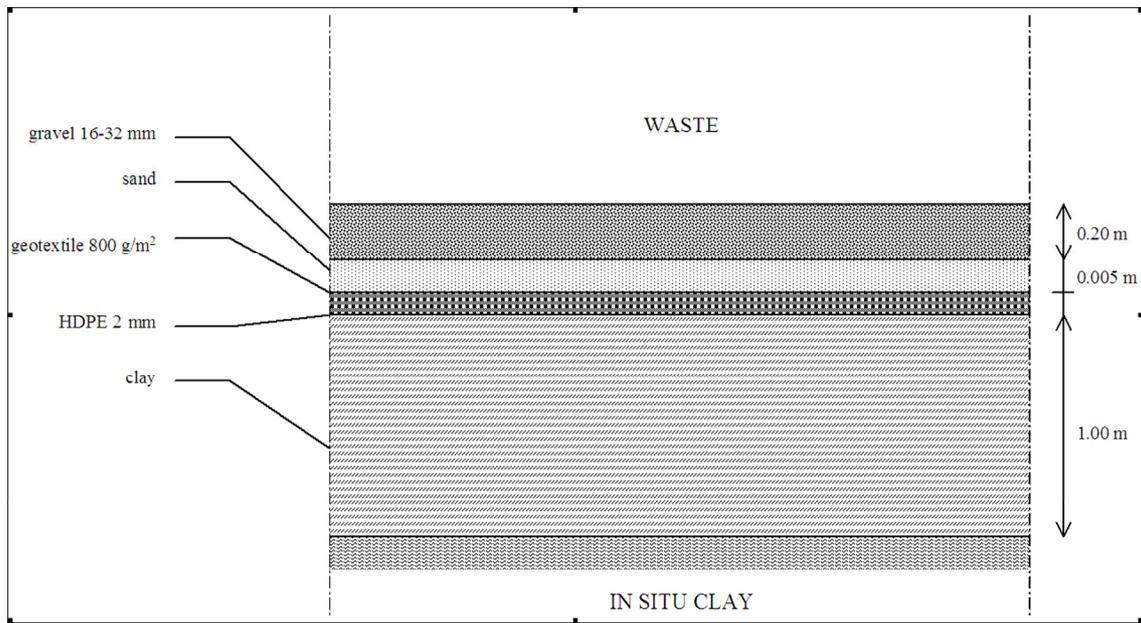


Figure 5. Example of a liner with more elements (system in parallel).

Table 1. Symbology used in the fault tree analysis.

EVENTS	
LOGIC ELEMENT SYMBOLS	MEANING
	Primary system
	Intermediate event
	Top Event or Final Event
LOGIC GATE	
LOGIC ELEMENT SYMBOLS	MEANING
	The event happens if E ₁ and E ₂ simultaneous take place
	The event happens if E ₁ or E ₂ takes place

For generic event E_i , the probability $P(E_i)$ is the existence probability of the event A at time t . Given two generic events A and B , each characterized by an actual probability, the following relations are verified:

$$P(E) = P(E_1 \text{ (and) } E_2) = P(E_1 E_2) = P(E_1 \cap E_2) = P(E_2) \cdot P(E_1 | E_2) = P(E_1) \cdot P(E_2 | E_1)$$

$$P(E) = P(E_1 \text{ (Or) } E_2) = P(E_1 + E_2) = P(E_1 \cup E_2) = P(E_1) + P(E_2) - P(E_1 \cap E_2)$$

where $P(E_1 | E_2)$ is the conditional probability of E_1 , given E_2 and it is equal to:

$$P(E_1 | E_2) = \frac{P(E_1 E_2)}{P(E_2)}$$

If E_1 and E_2 are independent the above expressions become easier, because $P(E_1 | E_2) = P(E_1)$. In the case of more events (E_1, E_2, E_3 and E_4) the probability of the top event is:

$$P(E) = P(E_1 \cap E_2 \cap E_3 \cap E_4) = P(E_1)P(E_3 | E_1 \cap E_2)P(E_4 | E_1 \cap E_2 \cap E_3)$$

$$P(E) = P(E_1 \cap E_2 \cap E_3 \cap E_4) = P(E_1) + P(E_2) + P(E_3) + P(E_4) - P(E_1 \cap E_2 \cap E_3 \cap E_4)$$

Knowing the probabilities of the individual basic events that constitute the system's Fault Tree, you can estimate the probability of failure of the entire system by means of these fundamental algebra rules.

A detailed Fault Free can be developed for the bottom liner of a Sanitary Landfill. The diagram structure should contain a mineral liner, a collection system and a synthetic liner. The failure of the whole liner system occurs in the case of simultaneous failure of the mineral liner (clay, bentonite), synthetic liner (geomembrane, GCL) and leachate collection system. The probability (P(E)) of liner failure can be determined as follows:

$$P(E) = P(E_1 \cap E_2 \cap E_3)$$

The events are dependent. In fact, a failure of one component increases the load supported by the other components. Consequently, the remaining components are more likely to fail, and we can not assume statistical independence of components.

The functionality loss of each of these three components is due to different causes that international literature has studied for a long time. Although each failure is an individual event related to site-specific ground conditions, climate conditions and design details, general behaviour trends can be deduced by considering these three elements. A summary of the findings is presented in **Table 2**. However, for each component a main failure state

can be defined as shown in **Table 3**. The failure of the component at time t occurs if the physical variable (pi) that describes the failure state is higher than a safety or project value (si).

4.2. Conditional Events

The calculation of safety or failure probability of a system through the above equations is generally difficult due to the dependence of variables; approximation is almost always necessary. With regard to the latter, upper bounds of the corresponding probabilities are useful under the conservative principle assumption.

For the selected fault tree, an estimation of the failure upper bound (P(E)) is [26]:

$$P(E) \leq 1 - \prod_{i=1}^3 [1 - P(E_i)]$$

This expression indicates that the containment system will survive until all the components (mineral liner, synthetic liner and collection system) will work. This is a strong simplification of the study, but at the moment, if there are not sufficient data to support the conditional statistics of the compartments, it is the only solution.

Table 2. Causes of the basic failure events.

COMPONENT	CAUSES
COLLECTION SYSTEM	Settlement, bad design and/or choice of materials, clogging due to particulate transport/chemical precipitation, Clogging due to biological material buildup, Pipe breakage/slope change
MINERAL LINER	Waste movement, settlement, bad compaction, bad design and/or choice of materials, pipes penetrating liner, geotechnical failure, uncontrolled groundwater infiltration, instability of the sub-grade both slope and basal heave, exhaustion adsorption capacity, increase in hydraulic conductivity due to interaction with leachate and to cracking
SYNTHETIC LINER	Installation damage, bad design and/or choice of materials, aging, pipes penetrating liner, geotechnical failure, unanticipated chemical attack, tension of the materials, uncontrolled groundwater infiltration, instability of the sub-grade both slope and basal heave

Table 3. Failure state for single component.

COMPONENT	DESCRIPTION OF FAILURE	PHYSICAL VARIABLE (p _i)	THE FAILURE STATE (s _i)	REFERENCE
Leachate collection system	Clogging of drainage layer due to chemical precipitation and to bio-film growth	Ks (Hydraulic conductivity)	10 ⁻⁵ - 10 ⁻⁷ m/s	[7,8]
	Exhaustion adsorption capacity	EC (Exchangeable Cations)	CEC (Cation Exchange Capacity)	[9]
Mineral liner	Increase in hydraulic conductivity due to interaction with leachate and to cracking	Ks (Hydraulic conductivity)	10 ⁻⁹ m/s	[10-12]
	Aging of matrix structure due to the corrosive effects of leachate and to elevated temperatures generated by the exothermic processes occurring in landfills	Concentration of antioxidant	Allowable number/type of defects as reported in the Construction Quality Assurance	[13-19]
Synthetic liner	Damage due to poor dumping practices	Number of defects by unit area	Allowable number/type of defects as reported in the Construction Quality Assurance	[20-25]

5. Liner Failure Data Base Procedure

When N items being considered fail respectively at times t_1, t_2, \dots, t_n , then the failure probability at time t_1 can be approximated by $F(t_1) = 1/N$, at time t_2 by $F(t_2) = 2/N$, and, in general by $F(t_r) = r/N$.

Given sufficient data, a failure distribution can be determined by a piecewise polynomial approximation. When only fragmentary data are available we cannot construct the complete curve. In such case, an appropriate distribution (such as Exponential, Normal, Log-Normal, Weibull, Poisson, etc.) must be assumed and its parameters evaluated from data.

This approach can be conducted in two different ways.

First, the failure data are related to many landfills where the failure has been ascertained by means of monitoring data (inductive analysis). The failure of the system has been indirectly estimated as chemical concentration (for example in a monitoring well outside the landfill) exceeding a table value. The problem of this approach consists in 1) the selection of a group of landfills with similar liner design and operating conditions; 2) scarce data available on groundwater contamination before the establishment of Law 471/99 in Italy; 3) unsuitable location of monitoring wells; 4) ambiguous data that does not permit locating the contaminant source; and, 5) underestimated failure curves, because it considers degradation of contaminants in the landfill, natural attenuation in liner and in the environment.

Second, the failure data are related to single component performance (mineral layer, drainage system, synthetic liner) according to **Table 3**. Probability re-implosion of the components results in failure of the entire system (predictive system). For these reliability problems, the 'average' failure data from several lab tests may best describe the system behaviour. In this case, measurements of a parameter at one scale (eg. laboratory measurements) can be used to define the parameter at a larger scale. This approach of using sample measurements to define the 'average' system behaviour is described as upscaling. Where the system is believed to be heterogeneous, then upscaling should be used with care.

However, literature studies reveal that field and lab data on landfill failures are not enough for establishing probability distributions. In the future, a more accurate measure of liner failure could be done by a monitoring approach based on a Leak Detection Sump [27]. Therefore, subjective data needed to be included. In these cases it has become fairly customary for experts in related fields to be asked to give their best subjective estimate, *i.e.* their expert opinion on the subject.

Direct estimates about the mean life of liner barrier components can be obtained by the Delphi technique the

contribution of each factor to the failure of the subsystem. The purpose of the Delphi technique is to elicit information and judgments from participants to facilitate the resolution of reliability problems when there are no field data. It does so without physically assembling the contributors. Instead, information is exchanged via mail, FAX, or email. This technique is designed to take advantage of participants' creativity as well as facilitating effects of group involvement and interaction. It is structured to capitalize on the merits of group problem-solving and minimize the liabilities of group problem-solving.

According to the first approach, a failure distribution has been determined for a size sample of almost 30 sites in the North of Italy that are designed as contained landfills respecting the following principles (details on landfills are collected in **Table 4**):

- Minimize rainfall infiltrations;
- Maintain anaerobic conditions;
- Isolate the waste from the environment with natural and artificial materials;
- Collect biogas and leachate by means of extraction systems, such as vertical and horizontal materials (when collection systems are present).

Figure 6 shows the failure of landfills in the first 30 years and the Herz model fitting curve [6]. The application shows that in the North of Italy landfills can contaminate with high probability (more than 60%) the groundwater in the first 30 years.

6. Conclusions

This paper illustrates a suitable methodology for evaluating landfill liner failure during aftercare. There are two different approaches: a deductive and a predictive analysis. The former can be used only for landfills with similar design and operating conditions, the latter (more flexible) requires information regarding correlation of variables. For successful application, both approaches require more accurate liner failure data.

Currently, the analysis of failure data shows a lack of information to assess the approach of system reliability. A simplification can be obtained considering the worst case ($P(E) = 1$) for the containment system. This assumption is routinely included in traditional hydrological risk assessments and it is reliable if the failure time is lower than the simulation time in which the risk is evaluated; otherwise the approach is too conservative and the results do not represent what really could occur.

In this "precautionary" approach, average defect values for synthetic liner are assumed; performance of mineral liner remains constant over time and is the same as measured in the liner test; performance of drainage system is indirectly considered in the leachate head estima-

Table 4. Characteristics of landfills used for the definition of the failure curve. All the landfills are sited in the North of Italy. For each landfill the failure time has been estimated as the number of years after the beginning in which the chemical concentration exceeding a table value. Municipal Solid Waste = MSW; Inert Waste = IW.

LANDFILL CODE	WASTE TYPE	CONTAINMENT SYSTEM DESIGN	LANDFILL VOLUME	ESTIMATED FAILURE TIME
			(m ³)	(years)
RSA	MSW	Clay liner (>1 m), geomembrane, drainage layer, leachate collection system	1,450,000	12
BCA	MSW	Clay liner (>1 m), geomembrane, drainage layer, leachate collection system	700,000	43
NBA	MSW	Clay liner (>1 m), geomembrane, drainage layer, leachate collection system	600,000	22
CAN	MSW	Clay liner (>1 m), geomembrane, drainage layer, leachate collection system	350,000	19
URB	MSW	Clay liner (>1 m)	200,000	1
GRI	MSW and IW	Clay liner (>2 m), geomembrane, drainage layer, leachate collection system	420,000	59
DEN	MSW	Clay liner (>1 m), geomembrane, drainage layer, leachate collection system	135,000	28
AUS	MSW	Clay liner (>1 m), drainage layer, leachate collection system	900,000	36
GER	MSW	Clay liner (>1 m), geomembrane	850,000	17
NOD	MSW	Clay liner (>1 m), geomembrane, drainage layer	930,000	34
USA	MSW	Clay liner (>1 m), drainage layer, leachate collection system	1,300,000	5
AMC	MSW	Clay liner (>1 m), drainage layer, leachate collection system	1,100,000	20
BBL	IW	Clay liner (>1 m), geomembrane	970,000	25
BST	MSW	Clay liner (>1 m) geomembrane, drainage layer, leachate collection system	780,000	26
BRT	MSW	Clay liner (>1 m) geomembrane, drainage layer, leachate collection system	670,000	25
ILP	MSW	Clay liner (>1 m), geomembrane	440,000	13
RIF	MSW	Clay liner (>1 m) geomembrane, drainage layer, leachate collection system	820,000	7
MCH	MSW	Clay liner (>1 m) geomembrane, drainage layer, leachate collection system	600,000	28
RNO	MSW and IW	Clay liner (>1 m), geomembrane	760,000	30
SHC	MSW and IW	Clay liner (>1 m) geomembrane, drainage layer, leachate collection system	300,000	23
UNM	MSW	Clay liner (>1 m), geomembrane	470,000	40
CPD	MSW	Clay liner (>1 m) geomembrane, drainage layer, leachate collection system	292,500	27
AQO	MSW	Clay liner (>1 m), geomembrane	300,000	35
MDA	MSW	Clay liner (>1 m), geomembrane, drainage layer, leachate collection system	1,000,000	36
LGO	IW	Clay liner (>1 m), geomembrane, drainage layer, leachate collection system	1,600,000	38
TRO	MSW	Clay liner (>1 m), geomembrane, drainage layer, leachate collection system	3,200,000	39
CRA	MSW	Clay liner (>1 m), geomembrane, drainage layer, leachate collection system	1,101,000	40
TRV1	MSW and IW	Clay liner (>1 m)	250,000	41
TRV2	MSW and IW	Clay liner (>1 m) geomembrane	450,000	21
TRV3	MSW and IW	Clay liner (>1 m), geomembrane	650,000	33

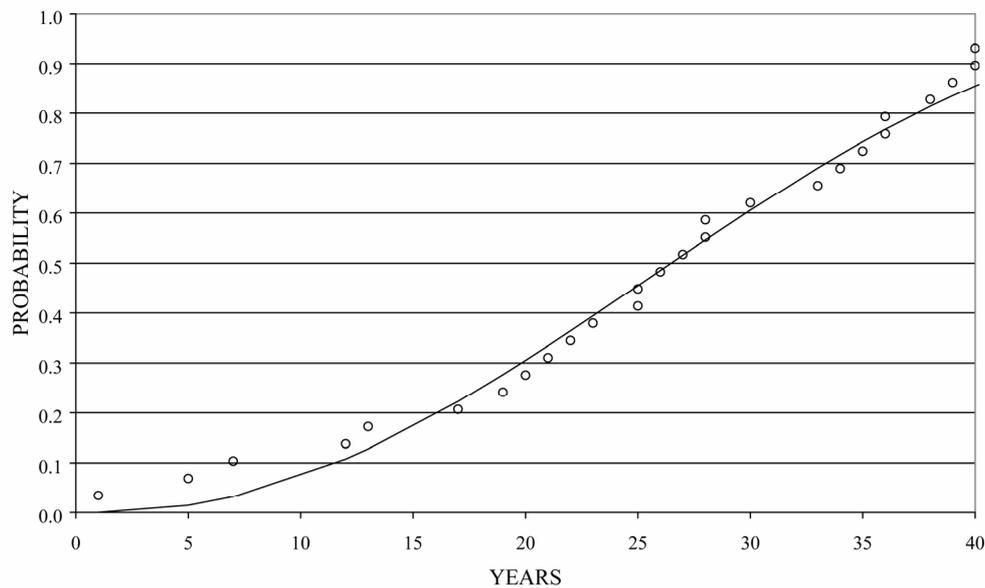


Figure 6. Cumulative curve of failure of contained landfills in the north of Italy.

tion used for assessing leachate leakage.

A simplification can be assumed considering the worst-case approach as is generally used in traditional hydrological risk assessments. This implies calculating the effects of contamination given that leachate has been released from the landfill liner. However, the results are often too conservative and do not represent what could actually occur.

7. Current & Future Developments

The approach described in the paper should be included in a standardized methodology in order to manage after-care period. Three should be the possible outcomes from this methodology:

Continue Aftercare. If leachate emissions still require significant levels of care within the regulatory framework for environmental protection, the outcome of the evaluation will direct continuation of aftercare under the currently approved plan. Some care activities may be optimized according to outcome of the study.

Optimize Aftercare. In many cases, the evaluation may reveal that the intensity or scope of some care activities can be reduced while still providing the necessary level of environmental protection. In these cases, the relevant aftercare activities may be optimized. Optimization may involve, for example, eliminating non-detected constituents from further monitoring, reducing maintenance frequencies, or changing the design of a system.

End Regulated Aftercare. If the study reveals that leachate emissions don't represent a risk for the environment, then regulated aftercare would be ended, al-

though a minimum level of care (herewith, custodial care) will invariably still be required (generally for the cap and general site upkeep). A custodial care program would involve property management activities that are typical of any property, such as paying property taxes, controlling access, complying with local zoning ordinances, and complying with the property-use restrictions identified in the deed to the property.

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